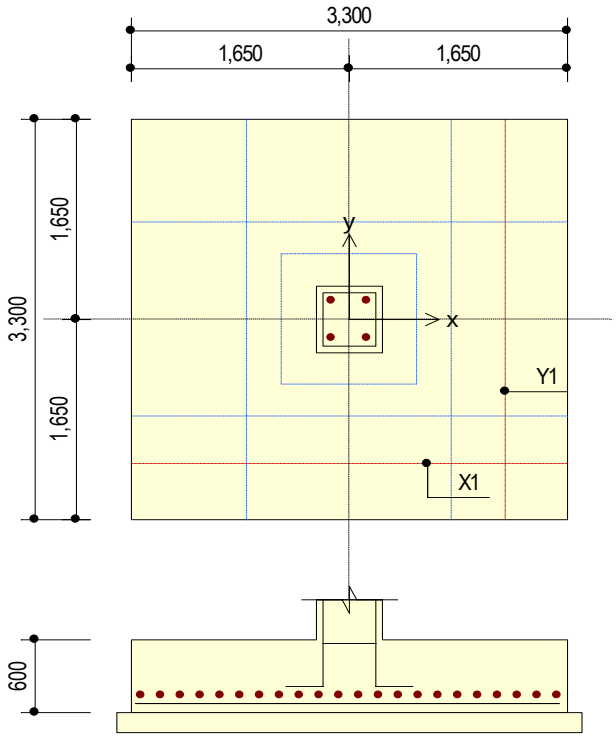
■ ZAPATA Z-1

1. General Information

(1) Design Code : ACI318M-14
(2) Unit System : N, mm

2. Material

(1) F'_c : 28.00MPa
(2) F_y : 400MPa



3. Section

(1) Section Size
- Depth : 600mm
- Cover : 75.00mm

(2) Column Section
- Shape of Column : Rectangle
- Section : 500x550mm

4. Rebar

(1) Direction Y

- Layer 1 : #5@150 (A_s = 1,333mm²)

(2) Direction X

- Layer 1 : #5@150 (A_s = 1,333mm²)

5. Foundation

(1) Foundation Size

- L_x : 3.300m
- L_y : 3.300m
- f_e : 120KPa

6. Design Load

(1) Service Load

- P_s : 983kN
- M_{sx} : 83.00kN·m
- M_{sy} : 64.00kN·m

(2) Factored Load

- P_u : 1,475kN
- M_{ux} : 125kN·m
- M_{uy} : 96.00kN·m

(3) Surcharge Load is not considered.

(4) Self weight is considered.

7. Check Soil Capacity

(1) Calculate actual soil stress (KPa)

- $q_{s.top\text{-}left}$ = 107 $q_{s.top\text{-}right}$ = 109
- $q_{s.bot\text{-}left}$ = 79.39 $q_{s.bot\text{-}right}$ = 101
- $q_{s.max}$ = 109
- qs.max / fe = 0.91 → O.K

(2) Calculate factored soil stress (KPa)

- $q_{u.top\text{-}left}$ = 157 $q_{u.top\text{-}right}$ = 189
- $q_{u.bot\text{-}left}$ = 115 $q_{u.bot\text{-}right}$ = 147
- $q_{u.max}$ = 189 $q_{u.min}$ = 115

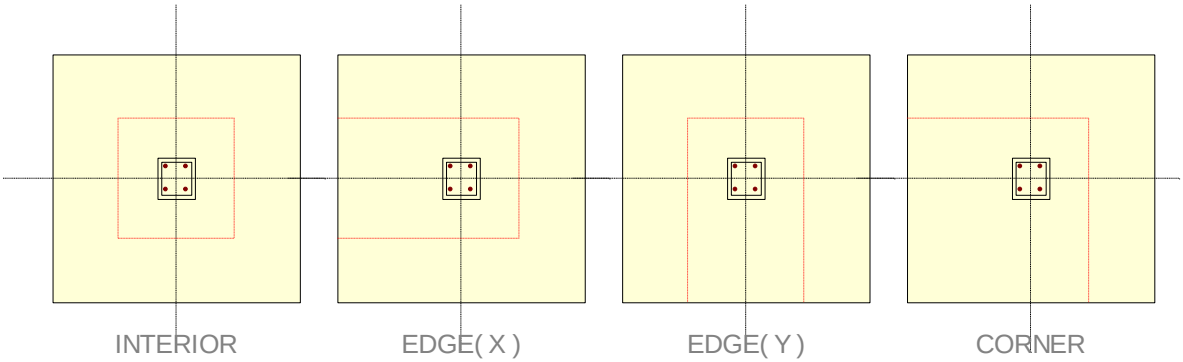
8. Check Shear

(1) Calculate one-way shear

- ø = 0.750
- V_{ux} = 401kN $øV_{cx}$ = 1,151kN
- V_{ux} / $øV_{cx}$ = 0.348 → O.K
- V_{uy} = 408kN $øV_{cy}$ = 1,116kN
- V_{uy} / $øV_{cy}$ = 0.365 → O.K

(2) Calculate two-way shear

-	b_0 (mm)	V_{c1} (kN)	V_{c2} (kN)	V_{c3} (kN)	V_c (kN)	V_u (kN)	$øV_c$ (kN)	Ratio
Interior	4,168	6,204	6,590	3,763	3,763	1,328	2,823	0.470
Edge(X)	5,384	8,014	5,968	4,861	4,861	1,143	3,646	0.314
Edge(Y)	5,384	8,014	5,968	4,861	4,861	1,156	3,646	0.317
Corner	4,342	6,463	4,320	3,920	3,920	777	2,940	0.264



- ø = 0.750
- d = 517mm
- $V_{c1} = 0.17\left(1 + \frac{2}{\beta}\right)\sqrt{f'_c}\ b_0\ d$
- $V_{c2} = 0.083\left(\frac{a_s\ d}{b_0} + 2\right)\sqrt{f'_c}\ b_0\ d$
- $V_{c3} = 0.33\sqrt{f'_c}\ b_0\ d$
- $V_c = \min(V_{c1}, V_{c2}, V_{c3})$
- V_u = 1,328kN
- $V_u / øV_c$ = 0.470 → O.K

9. Check Moment Capacity

(1) Calculate moment capacity (Direction X)

- ø = 0.900
- M_{uy} = 144kN·m $øM_{ny}$ = 243kN·m
- $M_{uy} / øM_{ny}$ = 0.593 → O.K

(2) Calculate moment capacity (Direction Y)

- ø = 0.900
- M_{ux} = 142kN·m $øM_{nx}$ = 235kN·m
- $M_{ux} / øM_{nx}$ = 0.605 → O.K

10. Check Rebar

(1) Calculate minimum rebar area required

- $A_{s.min}$ = 0.00180D = 1,080mm²

(2) Calculate minimum rebar space required (Direction Y)

- A_s = 200mm² (#5@150)
- $s_{req.}$ = 185mm
- s_x = 150mm < $s_{req.}$ = 185mm → O.K

(3) Calculate minimum rebar space required (Direction X)

- A_s = 200mm² (#5@150)
- $s_{req.}$ = 185mm
- s_y = 150mm < $s_{req.}$ = 185mm → O.K